

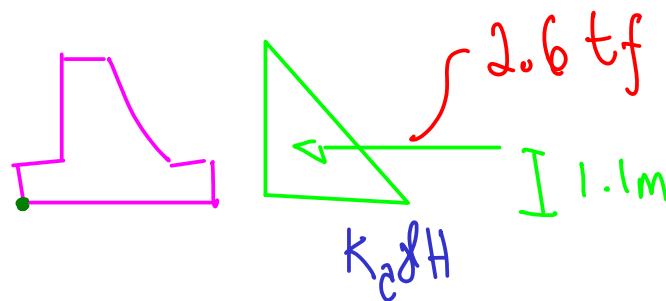
1: Determinación del coeficiente de empuje activo  $K_A$

$$K_A = \frac{1 - \operatorname{sen} \phi}{1 + \operatorname{sen} \phi} = \frac{1 - \operatorname{sen} 35}{1 + \operatorname{sen} 35} = 0.271$$

Empuje Activo

$$E_A = \frac{1}{2} K_A \gamma H^2 = \frac{1}{2} (0.271) \left( 1.762 \frac{\text{tf}}{\text{m}^3} \right) (3.3 \text{ m})^2$$
$$= 2.6 \text{ tonref}$$

Localización de  $E_A$

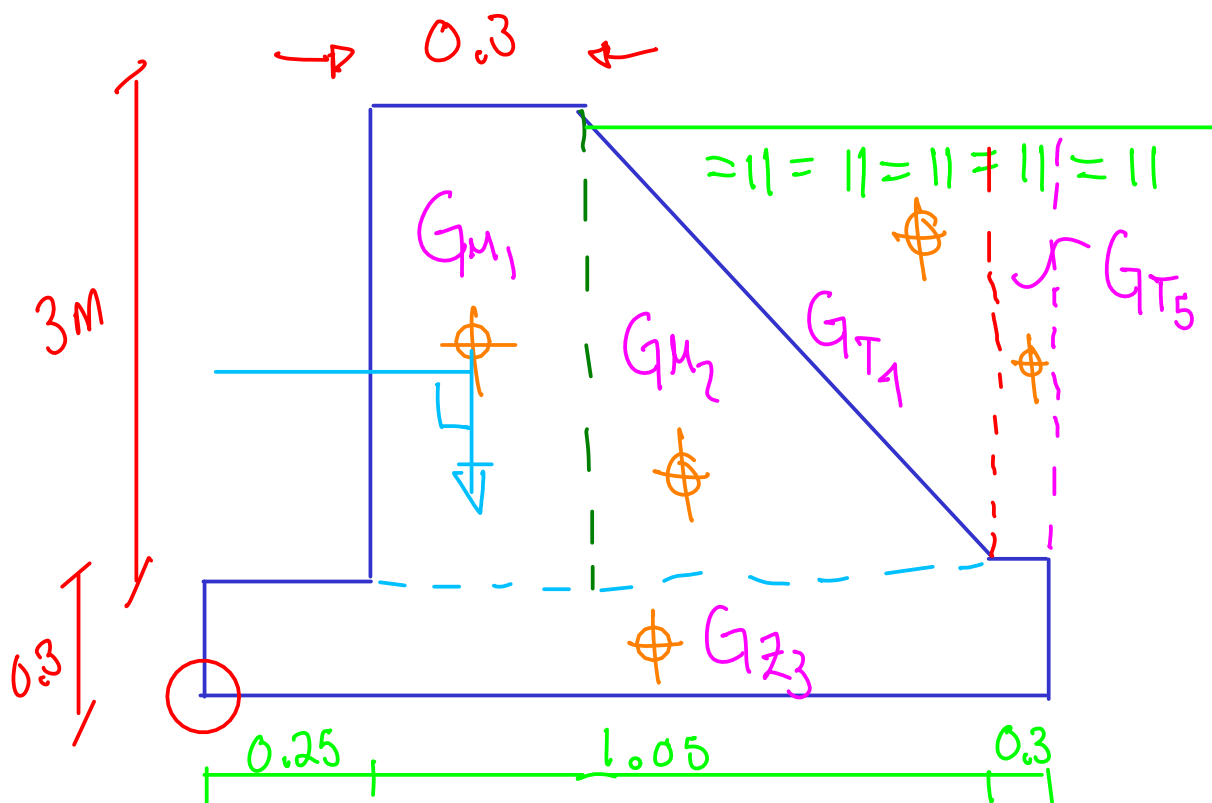


2: Momento de volteo

$$M_e = (2.6)(1.1) = 2.86 \text{ tfm} \nabla$$

3: Momento resistente

$M_i$



Peso (tonnef)      Brazo (m)      Momento

$G_{M1} = (3)(0.3)(2.32) = 2.088$	$0.25 + \frac{0.3}{2} = 0.4$	0.8352
$G_{M2} = \frac{1}{2}(0.75)(3)(2.32) = 2.61$	$0.25 + 0.3 + \frac{1}{3}(0.75) = 0.8$	2.088
$G_{M3} = (1.6)(0.3)(2.32) = 1.11$	$1.6/2 = 0.8$	0.89
$G_{T4} = \frac{1}{2}(0.75)(3)(1.762) = 1.98$	$0.25 + 0.3 + \frac{2}{3}(0.75) = 1.05$	2.08
$G_{T5} = (0.3)(3)(1.762) = 1.58$	$0.25 + 1.05 + 0.3/2 = 1.45$	2.299
		8.19
		9.38

4.- Factor de seguridad contra el volteo

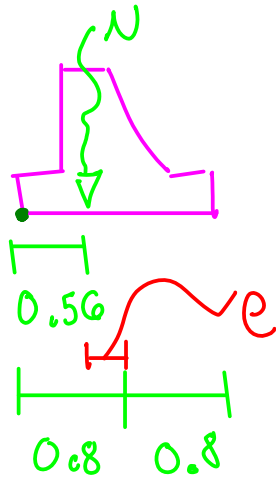
$$\frac{M_i}{M_e} = \frac{8.19}{2.86} = 2.87 > 2 \therefore \text{OK}$$

5.- Factor de seguridad contra el deslizamiento

$$F_{gr} = \mu N = 0.5 (9.38) = 4.69 \text{ tonnef}$$

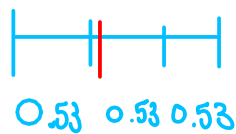
$$\frac{F_{gr}}{E_A} = \frac{4.69}{2.6} = 1.8 > 1.5 \therefore \text{OK}$$

6.- Presiones en el suelo



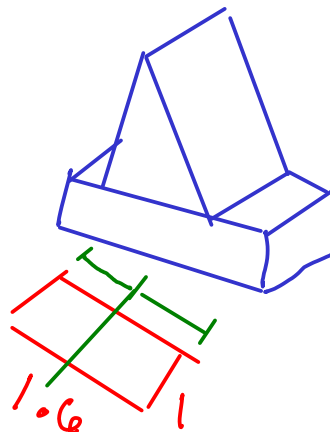
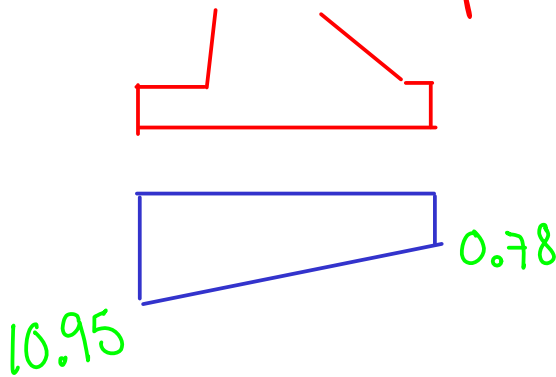
$$x = \frac{M_i - M_e}{N} = \frac{8.19 - 2.86}{9.38} = 0.56$$

$$e = \frac{b}{2} - x = \frac{1.6}{2} - 0.56 = 0.23$$



$$\frac{1.6}{3} = 0.53$$

∴ La resultante pasa por el tercio medio



$$M = N \cdot e$$

$$\sigma_1 = \frac{P}{A} + \frac{M_c}{I} = \frac{9.38}{(1.6)(1)} + \frac{(9.38)(0.23)(1.6/2)}{\frac{1}{12}(1)(1.6)^3} = 10.95 \text{ tf/m}^2$$

$$G_2 = \frac{P}{A} - \frac{M_c}{I} = 0.78 \text{ tf/m}^2$$

$q = 12.21 > 10.95 \therefore$  El suelo admite la presión.

$$\frac{12.21}{10.95} = 1.11 \text{ Fact. Seg. Aplastamiento}$$

7: Revisión deflexión en el concreto.